## Program: BE CIVIL Engineering

Curriculum Scheme: Revised 2016
Examination: Third Year Semester VI
Course Code: CEC602 and Course Name: Design and Drawing of Steel Structures
Time: 1 hour
Max. Marks: 50

Note to the students:- All the Questions are compulsory and carry equal marks .

| Q1. | For the steel of grade Fe $410,410 \mathrm{~N} / \mathrm{mm}^{2}$ is - |
| :--- | :--- |
| Option A: | Yield Stress |
| Option B: | Design stress |
| Option C: | Ultimate tensile stress |
| Option D: | Failure stress |
| Q2. | A tie member ISA $100 \times 75 \times 8$ with $\mathrm{A}_{\mathrm{g}}=16.50 \mathrm{~cm}^{2}$ connected with longer leg <br> using 5-M16 black bolts. Approximate rupture strength of member will be <br> nearly- |
| Option A: | 313 KN |
| Option B: | 320 KN |
| Option C: | 305 KN |
| Option D: | 330 KN |
| Q3. | Which of the following type of tension member is not mainly used in modern <br> practice |
| Option A: | open section such as angles |
| Option B: | Flat bars |
| Option C: | Double angles |
| Option D: | Circular section |
| Q4. | What is the effective length when both the end of compression member are <br> Hinged? |
| Option A: | 0.65 L |
| Option B: | 0.80 L |
| Option C: | 1.00 L |
| Option D: | 2.00 L |
| Q5. | The value of imperfection factor for a compression member for buckling class <br> "d" member is- |
| Option A: | 0.34 |
| Option B: | 0.45 |
| Option C: | 0.21 |
| Option D: | 0.76 |
|  |  |


| Q6. | Width of end batten in built-up column, when two channel sections are placed toe-to-toe is |
| :---: | :---: |
| Option A: | S+2g |
| Option B: | S-2g |
| Option C: | S+Cy ${ }_{\text {y }}$ |
| Option D: | S |
| Q7. | In case of Fillet Weld to calculate Size of weld, if the value of " $k$ is 0.55 " then the Angle of Fusion will be |
| Option A: | 60-90 degrees |
| Option B: | 91-100 degrees |
| Option C: | 101-106 degrees |
| Option D: | 107-113 degrees |
| Q8. | Under exactly identical conditions ,battened column as compared to laced column is |
| Option A: | Equal in strength |
| Option B: | Weaker in strength |
| Option C: | Stronger in strength |
| Option D: | 50\% stronger |
| Q9. | For very short compression member the design compressive stress $\mathrm{f}_{\mathrm{cd}}$ for Fe 410 grade steel is |
| Option A: | $166 \mathrm{~N} / \mathrm{mm}^{2}$ |
| Option B: | $250 \mathrm{~N} / \mathrm{mm}^{2}$ |
| Option C: | $240 \mathrm{~N} / \mathrm{mm}^{2}$ |
| Option D: | $227 \mathrm{~N} / \mathrm{mm}^{2}$ |
| Q10. | Depth of intermediate batten $=\ldots \ldots$ of depth of end batten |
| Option A: | 1/2 |
| Option B: | 4/3 |
| Option C: | 3/2 |
| Option D: | 3/4 |
| Q11. | Lacing shall be designed to resist transverse shear (Vt) equals to |
| Option A: | $0.5 \%$ of column load |
| Option B: | $2.5 \%$ of column load |
| Option C: | $5.0 \%$ of column load |
| Option D: | 8.0\% of column load |
| Q12. | In case of Plate Girder when there is second longitudinal stiffeners provided at neutral axis to meet serviceability criteria then, $\mathrm{d} / \mathrm{t}_{\mathrm{w}}$ should be, |
| Option A: | $\leq 230 \varepsilon_{\text {w }}$ |
| Option B: | $\leq 400 \varepsilon_{\text {w }}$ |
| Option C: | $\leq 340 \varepsilon_{\text {w }}$ |
| Option D: | $\leq 200 \varepsilon_{\text {w }}$ |
| Q13. | In case of Plate Girder, If Elastic Critical Stresses $\left(\tau_{\text {cr,e }}\right)=75 \mathrm{~N} / \mathrm{mm}^{2}$ fyw $=250$ $\mathrm{N} / \mathrm{mm}^{2}$, then the shear stress corresponding to the buckling $\left(\tau_{\mathrm{b}}\right)$ is |
| Option A: | $65.50 \mathrm{~N} / \mathrm{mm}^{2}$ |



| Q21. | Generally the purlins are placed at the panel points so as to allow only- |
| :--- | :--- |
| Option A: | Axial force in rafter |
| Option B: | Shear force in rafter |
| Option C: | Deflection of rafter |
| Option D: | Bending moment in rafter |
|  |  |
| Q22. | The self-weight of a roof truss of span 30 m can be taken as |
| Option A: | $75 \mathrm{~N} / \mathrm{m}^{2}$ |
| Option B: | $100 \mathrm{~N} / \mathrm{m}^{2}$ |
| Option C: | $150 \mathrm{~N} / \mathrm{m}^{2}$ |
| Option D: | $4000 \mathrm{~N} / \mathrm{m}^{2}$ |
|  |  |
| Q23. | The beam said to be laterally supported if- |
| Option A: | Tension flange is supported throughout |
| Option B: | Supported at both the ends only |
| Option C: | Compression flange is supported throughout |
| Option D: | Supported laterally at mid-span |
|  |  |
| Q24. | For a single I-section as a beam, the web buckling should be checked- |
| Option A: | At the junction of flange and web |
| Option B: | At the root of fillet of web |
| Option C: | At mid-point of flange |
| Option $\mathrm{D}:$ | At mid-depth of web |
|  |  |
| Q25. | When $\mathrm{V} \leq 0.6 \mathrm{Vd}$, then the design bending strength of beams is given by |
| Option A: | $\beta_{\mathrm{b}} / \mathrm{Z}_{\mathrm{p}} \mathrm{f}_{\mathrm{y}} \gamma_{\mathrm{m} 0}$ |
| Option B: | $\beta_{\mathrm{b}} \mathrm{Z}_{\mathrm{p}} \mathrm{f}_{\mathrm{y}} / \gamma_{\mathrm{m} 0}$ |
| Option C: | $\beta_{\mathrm{b}} \mathrm{Z}_{\mathrm{p}} / \mathrm{f}_{\mathrm{y}} \gamma_{\mathrm{m} 0}$ |
| Option D: | $\beta_{\mathrm{b}} \mathrm{Z}_{\mathrm{p}} \mathrm{f}_{\mathrm{y}} \gamma_{\mathrm{m} 0}$ |

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| Question | Correct Option (Enter either 'A' or 'B' or 'C' or 'D') |
| :---: | :---: |
| Q1. | C |
| Q2. | A |
| Q3. | B |
| Q4. | C |
| Q5. | D |
| Q6. | D |
| Q7. | D |
| Q8. | B |
| Q9. | D |
| Q10. | D |
| Q11. | B |
| Q12. | B |
| Q13. | C |
| Q14. | C |
| Q15. | B |
| Q16. | C |
| Q17. | B |
| Q18. | B |
| Q19. | D |
| Q20. | D |
| Q21. | A |
| Q22. | C |
| Q23. | C |
| Q24. | D |
| Q25. | B |

