Program: BE CIVIL Engineering

Curriculum Scheme: Revised 2012

Examination: Third Year Semester VI

Course Code: CEC602 and Course Name: Design and Drawing of Steel Structures

Time: 1 hour

Yielding

Rupture

Shear leg effect

Block shear

column is

Option B:

Option C:

Option D:

Q5.

Max. Marks: 50

Q1. Poisson's ratio of steel in elastic state is-Option A: 0.2 Option B: 0.35 Option C: 0.3 Option D: 0.4 Q2. Limiting slenderness ratio of a member carrying compressive loads resulting from dead loads and imposed loads is Option A: 180 Option B: 200 Option C: 350 Option D: 400 Q3. The value of imperfection factor for an angle compression member is taken as-Option A: 0.34 Option B: 0.45 Option C: 0.49 Option D: 0.76 Q4. Usually the design strength of a tension member is governed-Option A:

Under exactly identical conditions ,battened column as compared to laced

Note to the students:- All the Questions are compulsory and carry equal marks .

Option A:	Equal in strength	
Option B:	Weaker in strength	
Option C:	Stronger in strength	
Option D:	Data insufficient	
Q6.	Maximum spacing of lacing bars shall be such that the maximum slenderness of the main member between consecutive lacing connection is not more than	
Option A:	30	
Option B:	40	
Option C:	50	
Option D:	60	
Q7.	The outstand of gusset plate from the column flange edge is limited to because buckling consideration	
Option A:	9.4 &tg	
Option B:	16.6 &tg	
Option C:	8.4 Et _g	
Option D:	13.6 &t _g	
Q8.	Compression members always tend to buckle-	
Option A:	About major principle axis	
Option B:	About minor principle axis	
Option C:	About any lateral axis	
Option D:	About the polar axis	
Q9.	Lateral buckling in beam is	
Option A:	Non-dimensional	
Option B:	one dimensional	
Option C:	two dimensional	
Option D:	three dimensional	
Q10.	The value of ' β_b ' in the equation of design bending strength for plastic section is	
Option A:	1.5	
Option B:	2	
Option C:	0.5	
Option D:	1	

Q11.	Web crippling in a beam is initiated at -	
Option A:	mid depth of web	
Option B:	the junction of web and flange	
Option C:	the root of fillet in web	
Option D:	the midpoint of flange	
Q12.	As per IS Code the moment capacity of the trial section of a plate girder is calculated by-	
Option A:	$\beta_b \ Zp \ f_y \ / \ \gamma_{m1}$	
Option B:	$\beta_b \ Zp \ f_u \ / \ \gamma_{mb}$	
Option C:	$\beta_b \ Zp \ f_y \ / \ \gamma_{m0}$	
Option D:	$\beta_b \ Zp \ f_u \ / \ \gamma_{m1}$	
Q13.	In case of plate girder, to improve the buckling strength of a slender web due to shear, which stiffener is provided?	
Option A:	Bearing stiffeners	
Option B:	Diagonal stiffeners	
Option C:	Intermediate transverse web stiffeners	
Option D:	Load carrying stiffeners	
Q14.	If the end bearing Stiffeners of a welded plate Girder having Slenderness ratio λ = 20, fy=250 N/mm ² , & buckling curve = 'c', Calculate the buckling resistance (P _d) of the end bearing stiffeners, if effective area of the stiffener is 15500 mm ² ,	
Option A:	3672 kN	
Option B:	3202 kN	
Option C:	3172 kN	
Option D:	3472 kN	
Q15.	Web of the Plate Girder is designed to resist	
Option A:	Tension in Web	
Option B:	Torsion	
Option C:	Shear	
Option D:	Bending moment	
Q16.	The section ISMB 350 @ 52.4 Kg/m to be used in flooring system is	
Option A:	Plastic section	

Option B:	Compact section	
Option C:	Semi-compact section	
Option D:	Slender section	
Q17.	Which of the following is the reason for beams, plate girders and columns being spliced-	
Option A:	due to limitations of fabrication shop	
Option B:	For easy transportation	
Option C:	For aesthetic appearance	
Option D:	For frictional resistance	
Q18.	Seat angle used in an unstiffened seat connection is designed for-	
Option A:	Only bearing	
Option B:	Only shear	
Option C:	Only bending moment	
Option D:	Bearing, shear and bending moment	
Q19.	The slope of a roof truss is 30° (access is not provided except maintenance), the imposed load on the roof truss taken as	
Option A:	350 N/m^2	
Option B:	400 N/m ²	
Option C:	750 N/m ²	
Option D:	1500 N/m ²	
Q20.	The self-weight of a roof truss (N/m^2) may be obtained	
Option A:	(span/3+5)×10	
Option B:	(span/5+3)×10	
Option C:	(span/3-5)×10	
Option D:	(span/5-3)×10	
Q21.	The purlins in the roof trusses are subjected to unsymmetrical bending because the loading	
Option A:	is parallel to the minor principal axis, but does not coincide.	
Option B:	is perpendicular to the minor principal axis.	
Option C:	is inclined to the minor principal axis.	

Option D:	coincide with the minor principal axis.	
Q22.	While designing a steel roof truss, the IS Code 875 Part-IV is referred for-	
Option A:	Live load	
Option B:	Earthquake load	
Option C:	Load Combination	
Option D:	Snow load	
Q23.	Which of the following is a disadvantage of Steel?	
Option A:	High strength per unit mass	
Option B:	High durability	
Option C:	Fire and corrosion resistance	
Option D:	Reusable	
Q24.	In a bolted bracket connection Type-II, the most critical bolt is subjected to-	
Option A:	Only shear	
Option B:	Only tension	
Option C:	Only bearing	
Option D:	Combined shear and tension	
Q25.	In a given flooring system, if the secondary beam carries total distributed load as 200 KN (factored) then the beam end connection will be designed for a shear of-	
Option A:	100 KN	
Option B:	150 KN	
Option C:	175 KN	
Option D:	200 KN	

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Question	Correct Option
	(Enter either 'A' or
	'B' or 'C' or 'D')
Q1.	С
Q2.	Α
Q3.	С
Q4.	Α
Q5.	В
Q6.	С
Q7.	D
Q8.	В
Q9.	D
Q10.	D
011	С
Q11.	
Q12.	C
Q13.	С
Q14.	D
Q15.	С
Q16.	Α
Q17.	В
019	D
Q18.	D
Q19.	В
Q20.	A
Q21.	C
Q22.	D
Q23.	С
Q24.	D
Q25.	Α